# Chapter 4 <u>Bridge Program Drawings</u>

## **Section 4.19-Reinforcing Steel**

### Introduction

Because concrete has a low tensile strength, reinforcing steel is embedded in the concrete to resist the applied loads causing tension. Compressive reinforcing steel can be used in beams and columns to reduce the size of these members. In addition, reinforcing steel can control strain and subsequent concrete cracking from temperature and shrinkage and distribute applied loads to other reinforcing steel.

Reinforcing steel is in the form of bars or wires whose surface may be coated, smooth, or deformed. Deformed bars are generally used because they produce better bond with the concrete due to the raised patterns on the steel. Smooth bars are generally used in welded wire fabric.

Reinforcing Steel Types

**DEFORMED BARS** range in diameter from  $\frac{3}{8}$ " to  $2\frac{1}{4}$ ". Bar sizes are designated by a number, which is approximately eight times the bar diameter. The most commonly used bars are No. 4 through No. 11 bars, with No. 3 bars being limited to use as spiral reinforcing in columns. Reinforcing steel shall have yield strength of 60,000 psi (Grade 60).

**WELDED WIRE FABRIC** consists of a rectangular grid of uniformly spaced smooth wires, welded at their intersections. Welded wire fabric offers fast and easy field placement.

General Design and Detail Information

**REINFORCING STEEL COVER** refers to the clear distance from the concrete surface to the face of the reinforcing steel. Minimum clearances must be maintained to protect the reinforcing steel from corrosion and to allow for proper concrete placement. Concrete cover to the face of reinforcing steel shall be 2" unless noted. It is not necessary to provide any cover from an internal construction joint. The end of the bar may be placed against or rest on the joint. Other exceptions include the following.

> 4.19 - 1 Apr 2007 Rev Nov 2018

Curb, sidewalk, and barrier rail contraction joint	1"
Drilled hole to face of reinforcing	1"
Bridge decks - top of deck - bottom of deck	2½" 1"
Curbs and sidewalks - top and sides Bridge barrier rail- sides	11⁄2"
Any concrete surface resting on or placed against earth - bottom of spread footing - bottom of bottom slab of culvert - bottom of abutment wingwall - bottom of drilled shafts	3" 3" 3" 0"
T-girder and box girder webs - bottom face and sides Bottom slab of box girder- top and bottom faces Column - diameter < 48" Bottom of approach slabs Bottom of slope paving	11/2"
Drilled shaft foundation, side clearance	3" (Min)
Concrete cover on precast concrete sections shall confo	rm to

Concrete cover on precast concrete sections shall conform to AASHTO requirements.

Reinforcing steel is identified on the Bridge Program drawings by **BAR MARKS**. Bar marks identify each bar by designating the bar size and length in the case of a straight bar and the bar size, structural unit, and unique number for a specific unit of the structure for bent bars. The unique number for each specific unit shall be numbered consecutively.

Straight bars are marked as follows.

Mark	<b>ASTM Bar Size</b>	Length
405-10	No. 4	5'-10"
817-3	No. 8	17'-3"
1025-6	No. 10	25'-6"

Bent bars (ties, stirrups, cranks, etc,) are marked as follows.

Mark	<b>ASTM Bar Size</b>	Designation
4B4 (Tie)	No. 4	B4
7A3 (Stirrup)	No. 7	A3
5S1	No. 5	SI
Spiral 3B3	No. 3	B3

The following letters shall be used as standard designators for each structural unit.

- A: Abutment
- AS: Approach Slab
- B: Bent/Pier
- C: Reinforced Concrete Box Culvert
- G: Concrete Girder
- R: Concrete Bridge Barrier Rail
- S: Slab
- W: Retaining Wall

When the word **TIE** or **STIRRUP** follows a bent bar designation, it refers to the type of bent bar. Closed bent bars should be designated as ties, while open bent bars should be designated as stirrups. The word tie or stirrup informs the fabricator that minimum bends are required. Lengths of bent bars are shown in the Bill of Reinforcement.

The following table shows the American Concrete Institute standards for hooks. Any hook shown on the plans and not otherwise dimensioned shall conform with the hooks shown in this table.

STAND	ARD HO	OKS	- GRAD	E 60		
	END	HOO	к			
	Ban			90° HOOK	180°	ноок
Detailing Dim	Bar Size	d	D	Hook A or G	Hook A or G	J
	# 3	1 " 8	2 <u>1</u> "	6"	5"	3"
4 or 6	# 4	1 = = 2	3"	8"	6"	4"
	# 5	<u>5</u> "	3 <u>3</u> "	10"	7"	5"
<u>90° HOOK</u>	# 6	<u>3</u> "	4 <u>1</u> "	1'-0"	8"	6"
	# 7	<u>7</u> "	5 <u>1</u> "	1'-2"	10"	7"
Detailing Dim Hook A or G	# 8	1"	6"	1'-4"	11"	8"
	# 9	1븝"	9"	1'-7"	1'-3"	11 <u>1</u> "
	# 10	14"	10 <u>1</u> "	1'-10"	1'-5"	1'-0 <del>3</del> "
4d or 2 <sup>1</sup> / <sub>2</sub> " (Min)	# 11	1 <u>3</u> "	11 <u>4</u> "	2'-0"	1'-7"	1'-2 <sup>1</sup> / <sub>4</sub> "
180° HOOK	# 14	1 <u>3</u> "	1'-5"	2'-7"	2'-2"	1'-8 <sup>1</sup> / <sub>2</sub> "
<u>·····</u>	# 18	2 <sup>1</sup> / <sub>4</sub> "	1'-10 <sup>3</sup> 4"	3'-5"	2'-11"	2'-3"
s	TIRRUP	& TI	E HOOK			
	Bar Size		_	90° HOOK	135°	ноок
		d	D	Hook A or G	Hook A or G	н
	# 3	<u>3</u> "	1 <u>1</u> "	4"	4"	2 <sup>1</sup> / <sub>2</sub> "
	# 4	1 = 2	2"	4 <u>1</u> "	4 <u>1</u> "	3"
	# 5	<u>5</u> " 8	2 <sup>1</sup> / <sub>2</sub> "	6"	5 <u>1</u> "	3 <sup>3</sup> / <sub>4</sub> "
	# 6	<u>3</u> "	4 <u>1</u> "	1'-0"	8"	4 <u>1</u> "
	# 7	<u>7</u> "	5 <u>1</u> "	1'-2"	9"	5 <sup>1</sup> / <sub>4</sub> "
	# 8	1"	6"	1'-4"	10 <sup>1</sup> / <sub>2</sub> "	6"
Detailing Dim	2ġ"(Min)		Detail: ↓ ↓		Hook or G	
<u>90° STIRRUP HOOK</u> (Ties similar)				TIRRUP H ies similar)		

Reinforcing steel shall be **BILLED** in either the plan or elevation and called out in the section. When circumstances dictate, it may be necessary to bill the bar in the section. Billing is the method by which the required number, or count, of any bar is determined. Each bar shall be billed only once, but may be called out more than once elsewhere on the plans. The billing of bars is determined from the spacing for a particular group of bars.

Billing	Number Required
747-2 @ 3 Spa	4 Bars
405-10 @ 12"=22'-0"	23 Bars
7A3 @ 6"=15'-6" (T&B)	64 Bars (32 top face and
	32 bottom face)
817-3 @ 12"=9'-0" (EF)	20 Bars (10 each face)
609-8 @ 10 Eq Spa=9'-6"	11 Bars
4B4 @ 24"=12'-0"(4 per line)	28 Bars
5B3 @ 6"=4'-6" (Pairs)	20 Bars

The maximum bar length is 60'-0".

**BAR SPACING** shall be shown in inches, with the distance between the two end bars for any mark totaled and shown in feet and inches. When the first bar of any mark is referenced from a concrete edge and the distance is greater than 1'-0", that distance may be shown in feet and inches.

**SPIRALS** are lateral reinforcement used in columns and drilled shafts and are generally a No. 3 or No. 4 bar bent at a specific spacing or pitch. Vertical bars, or channel spacers, are furnished with each spiral to hold the spiral in place during placing of concrete. The core dimension for a spiral is the outside diameter of the reinforcement. Spirals require additional information for fabrication. The following data is generally shown in the Bill of Reinforcement.

Spiral 3B2: Pitch 3", Core 33", Turns 103

For columns, the number of turns is equal to the length of the column plus the distance from the top of the column to the bottom steel in the cap, divided by the pitch, plus one and one-half turns (top and bottom) required for development. For drilled shaft foundations, the number of turns is equal to the length of the drilled shaft foundation divided by the pitch, plus one and one-half turns (top only) for development. The number of turns shall be rounded up to the nearest one-half turn. A note shall be included on the sheet with the Bill of Reinforcement stating the number of

turns required top and/or bottom. The table shown in Chapter 5 -Quantities can be used to calculate the weight of spiral reinforcement per vertical foot of column or drilled shaft.

Often, with sloping surfaces and uniformly spaced bars, each bar length will be different. Rather than calling out each individual bar, a **SET** may be employed. Each group of bars will require a unique number identifying the set, with the first and last bars being called out. A set will be billed in the following manner.

Set 1 Bars @ 6" =12'-6"

Bent bars are detailed to the nearest  $\frac{1}{4}$ " out to out with the total length rounded up to the whole inch. Straight bars are detailed to the nearest whole inch. The average length of set bars shall be to the nearest  $\frac{1}{2}$ ". Because reinforcing steel is stocked in standard lengths, bars shall not be detailed greater than 60'-0". In bar runs longer than 60'-0", utilize 60'-0" long bars where possible. When a required bar length is longer than 60'-0", a lap is required. Bars shall be detailed using the maximum bar lengths, and one bar of lesser length placed within the run. The following table shows the dimensions and information required for commonly used bent bars.



Omit A or G dimensions for hooks (See Standard Hooks table).
Omit dimension on opposite side of symmetrical bars.
Omit run (longest) dimension of bevel only if bevel is 1:1.
Omit bevel dimensions on 135° stirrup hooks.
Omit curved length of bars where radius is shown.

- 6 Omit length of bend on crank bars.
- (7) Omit all repetitive dimensions on symmetrical crank bars.

When calculating bar lengths, consideration should be given to other bars lying in the same plane as the bar whose length is being calculated.



When bar lengths in a group or run require various lengths, rather than using a set, a uniform bar length may be employed to allow the lap between bars to be greater than the required minimum in order to facilitate fabrication and construction.

Reinforcing steel must be **DEVELOPED** for a sufficient length before it can achieve its design strength. This can be accomplished by either **LAPPING** it with other reinforcing steel, or **EXTENDING** it in each direction beyond the critical section. Lap and extension lengths vary depending on the bar size, cover, spacing, location, and concrete strength.

Under AASHTO Standard Specifications, the following shall apply:

A Class C lap is used for most cases. Laps other than Class C shall be designated in the design.

Top bar laps are used for horizontal reinforcing steel when a minimum of 12" of new concrete is placed below the bars.

Class A laps are used in regions of low tensile stress (where tensile reinforcement provided is more than twice that required for strength), if no more than three-quarters of the bars are lapped within a required lap length.

Class B laps are used in regions of low tensile stress (where tensile reinforcement provided is more than twice that required for strength) in regions where more than three-quarters of the bars are lapped within a required lap length; or in regions of high tensile stress (where tensile reinforcement provided is equal to or less than twice that required for strength), if no more than one-half of the bars are lapped within a required lap length.

Class C laps are used in regions of high tensile stress (where tensile reinforcement provided is equal to or less than twice that required for strength), if more than one-half of the bars are lapped within a required lap length.

Tabular values shall not be used in cases of bundled bars, welded splices, positive connections, or for reinforcement in lightweight concrete.

Lap lengths for bars enclosed and lapped inside of a spiral ("lap enclosed in spiral" in the following table) are based on Class C tension lap requirements.

Under AASHTO LRFD Bridge Design Specifications, the following shall apply:

The reinforcing steel table is based on Class B lap. Class B laps shall be used at all locations unless specified in the design.

Top bar laps are used for horizontal reinforcing steel when a minimum of 12" of new concrete is placed below the bars.

The following detail may be used as a guide to determine the correct lap length found in the reinforcing steel tables.



An epoxy coating is used to protect the reinforcing steel from corrosive actions. Epoxy coating shall be used on all bars billed with the bridge deck and curbs, sidewalks, raised medians, barrier rails, and approach slabs.

The following tables list the reinforcing steel properties, laps, and development length to be used for both epoxy coated and uncoated reinforcing steel.

The tables used shall match the design specification used. In general, the AASHTO Standard Specifications will be used for all bridge widening projects and bridge rehabilitation projects. The AASHTO LRFD tables will be used for all new structures.

#### AASHTO Standard Specifications for Highway Bridges 17th Edition - 2002

UN	COAT	ED B	AR PRO	OPERT	TIES, L	APS,	AND [	DEVEL		ENT L	ENGT	HS	
	GRADE 60												
		Bar Siz	e		#4	<b>#</b> 5	#6	#7	#8	#9	#10	#11	
		Weight			0.668 plf	1.043 plf	1.502 plf	2.044 plf	2.670 plf	3.400 plf	4.303 plf	5.313 plf	
		Diamete	r		0.5 in	0.625 in	0.75 in	0.875 in	1.00 in	1.128 in	1.27 in	1.41 in	
		Area			0.20 in <sup>2</sup>	0.31 in <sup>2</sup>	0.44 in <sup>2</sup>	0.60 in <sup>2</sup>	0.79 in²	1.00 in <sup>2</sup>	1.27 in <sup>2</sup>	1.56 in <sup>2</sup>	
	6"		Extension	Comp	0'-10"	1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"	
	AI	General	Extension	Tens	1'-0"	1'-3"	1'-6"	1'-9"	2'-1"	2'-8"	3'-4"	4'-1"	
	bu	ane	Tens Lap	ens Lap - Class C *		1'-9"	2'-1"	2'-8"	3'-7"	4'-6"	5'-8"	7'-0"	
	aci	ő	Comp Lap *		1'-3"	1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"	
psi	Spacing		Lap Enclos	Lap Enclosed in Spiral		1'-4"	1'-7"	2'-0"	2'-8"	3'-4"	4'-3"	5'-3"	
ă	Bar	Top Bar		Extension		1'-5"	1'-9"	2'-3"	2'-11"	3'-8"	4'-8"	5'-9"	
4000	ä	л В	Tens La	p - Class C	1'-11"	2'-5"	2'-11"	3'-9"	4'-11"	6'-3"	7'-11"	9'-9"	
	6"		Extension	Comp	0'-10"	1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"	
= 2	v	General	Extension	Tens	1'-0"	1'-3"	1'-6"	2'-0"	2'-7"	3'-4"	4'-2"	5'-2"	
r-	бu	sne	Tens Lap	- Class C *	1'-9"	2'-2"	2'-7"	3'-4"	4'-5"	5'-7"	7'-1"	8'-8"	
	aci	ð		Comp Lap *	1'-3"	1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"	
	Spacing		Lap Enclos	ed in Spiral		1'-8"	1'-11"	2'-6"	3'-4"	4'-2"	5'-4"	6'-6"	
1	Bar	Top Bar		Extension		1'-9"	2'-2"	2'-9"	3'-8"	4'-7"	5'-10"	7'-2"	
	Ë	Ъ́Я	Tens La	p - Class C	2'-5"	3'-0"	3'-7"	4'-8"	6'-2"	7'-10"	9'-11"	12'-2"	

\* Not for bars enclosed in a spiral Class A Lap = 59% of Class C Lap Class B Lap = 77% of Class C Lap

E	POXY	COA	TED E	BAR PF	ROPER	TIES,	LAPS,	AND D	DEVEL	OPME	NT LE	NGTH	S
	GRADE 60												
		Bar	Size			#4	#5	#6	#7	#8	#9	#10	#11
			eight			0.668 plf	1.043 plf	1.502 plf	2.044 plf	2.670 plf	3.400 plf	4.303 plf	5.313 plf
		Dia	meter			0.5 in	0.625 in	0.75 in	0.875 in	1.00 in	1.128 in	1.27 in	1.41 in
		A	rea			0.20 in <sup>2</sup>	0.31 in <sup>2</sup>	0.44 in <sup>2</sup>	0.60 in <sup>2</sup>	0.79 in <sup>2</sup>	1.00 in <sup>2</sup>	1.27 in <sup>2</sup>	1.56 in <sup>2</sup>
		6"		Extension	Comp		1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"
		A1	General		Tens	1'-0"	1'-2"	1'-5"	1'-10"	2'-5"	3'-1"	3'-10"	4'-9"
		bu	ane		- Class C *	1'-7" 1'-3"	2'-0"	2'-5"	3'-1"	4'-1"	5'-2"	6'-6"	8'-0"
	6db	Spacing	Ő		Comp Lap *		1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"
psi	and ≥6d	ds d		Lap Enclos	ed in Spiral		1'-6"	1'-10"	2'-4"	3'-1"	3'-10"	4'-11"	6'-0"
<u>e</u>	db g	Bar	Top Bar		Extension	1'-4"	1'-8"	2'-0"	2'-7"	3'-4"	4'-3"	5'-5"	6'-7"
4000	6 G	B	цщ	Tens La	p - Class C		2'-9"	3'-4"	4'-4"	5'-8"	7'-2"	9'-1"	11'-2"
4	spa	.9	_	Extension	Comp		1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"
, i		<u>×</u>	era.		Tens		1'-6"	1'-9"	2'-4"	3'-0"	3'-10"	4'-10"	5'-11"
<u>ب</u>	Cov	ju	General		- Class C *	2'-0"	2'-6"	3'-0"	3'-10"	5'-1"	6'-5"	8'-2"	10'-0"
	Ö	ac	Ğ		Comp Lap *	1'-3" 1'-6"	1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"
	- $\hat{\mathbf{O}}$ $\tilde{\mathbf{E}}$ $\tilde{\mathbf{D}}$ $\tilde{\mathbf{E}}$ $\tilde{\mathbf{E}}$ Tens Lap - Class $\hat{\mathbf{O}}$ $\hat{\mathbf{O}}$ Comp La $\hat{\mathbf{O}}$ Lap Enclosed in Spi						1'-10"	2'-3"	2'-11"	3'-10"	4'-10"	6'-1"	7'-6"
		Bar	⊖ kar ⊢ 00 Tens Lap - Class			1'-8"	2'-1"	2'-5"	3'-2"	4'-2"	5'-4"	6'-9"	8'-3"
		E E		l iens La	ıp - Class C	2'-9"	3'-6"	4'-2"	5'-5"	7'-1"	9'-0"	11'-5"	14'-0"

#### AASHTO Standard Specifications for Highway Bridges 17th Edition - 2002

\* Not for bars enclosed in a spiral Class A Lap = 59% of Class C Lap Class B Lap = 77% of Class C Lap

E	EPOXY COATED BAR PROPERTIES, LAPS, AND DEVELOPMENT LENGTHS												S
						GRA	DE 60						
		Bar	Size			#4	#5	#6	#7	#8	#9	#10	#11
		W	eight			0.668 plf	1.043 plf	1.502 plf	2.044 plf	2.670 plf	3.400 plf	4.303 plf	5.313 plf
		Dia	meter			0.5 in	0.625 in	0.75 in	0.875 in	1.00 in	1.128 in	1.27 in	1.41 in
		A	rea			0.20 in <sup>2</sup>	0.31 in <sup>2</sup>	0.44 in <sup>2</sup>	0.60 in <sup>2</sup>	0.79 in <sup>2</sup>	1.00 in <sup>2</sup>	1.27 in <sup>2</sup>	1.56 in <sup>2</sup>
		.9		Extension	Comp		1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"
		Ā	al		Tens		1'-6"	1'-10"	2'-5"	3'-2"	4'-0"	5'-0"	6'-2"
		or < 6db Spacing	General	Tens Lap - Class C *		2'-1"	2'-7"	3'-1"	4'-0"	5'-4"	6'-8"	8'-6"	10'-5"
	cdb 6db				Comp Lap *	1'-3"	1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"
psi	- 5 <u>9</u>	<u>ě</u>		Lap Enclos	ed in Spiral	1'-7"	1'-11"	2'-4"	3'-0"	4'-0"	5'-0"	6'-5"	7'-10"
ä	3db Ing	Bar	Top Bar		Extension	1'-5"	1'-9"	2'-1"	2'-8"	3'-7"	4'-6"	5'-8"	7'-0"
4000	ei 3	B	Ъщ	Tens La	ıp - Class C	2'-4"	2'-11"	3'-6"	4'-7"	6'-0"	7'-7"	9'-8"	11'-10"
	ver < 3db Spacing	.9		Extension	Comp	0'-10"	1'-1"	1'-3"	1'-6"	1'-8"	1'-11"	2'-1"	2'-4"
" 2		v	General	Extension	Tens	1'-6"	1'-11"	2'-3"	3'-0"	3'-11"	4'-11"	6'-3"	7'-8"
<u>ب</u>	Clear	Bu	ane -	Tens Lap	- Class C *	2'-7"	3'-3"	3'-10"	5'-0"	6'-7"	8'-4"	10'-7"	13'-0"
	C C C C C C C C C C C C C C C C C C C				Comp Lap *	1'-3"	1'-7"	1'-11"	2'-3"	2'-6"	2'-10"	3'-3"	3'-7"
1		ġ		Lap Enclos	ed in Spiral	1'-11"	2'-5"	2'-11"	3'-9"	5'-0"	6'-3"	8'-0"	9'-9"
	Bar S		Top Bar		Extension		2'-2"	2'-7"	3'-4"	4'-5"	5'-7"	7'-1"	8'-8"
		Ř	T, Bi	Tens La	ıp - Class C	2'-11"	3'-8"	4'-5"	5'-8"	7'-6"	9'-6"	12'-0"	14'-9"

\* Not for bars enclosed in a spiral Class A Lap = 59% of Class C Lap Class B Lap = 77% of Class C Lap

Jnco	ated	Bars		Grade 60							
			Bar Size	#4	#5	#6	#7	#8	#9	#10	#11
Minimum Clear Cove				1.000 in	1.250 in	1.500 in	1.750 in	2.000 in	2.250 in	2.625 in	2.625 in
Minimum Bar Spacing (C-C)				2.500 in	3.125 in	3.750 in	4.375 in	5.000 in	5.628 in	6.520 in	6.660 in
		Development	Comp	0' - 10"	1' - 0''	1' - 3"	1' - 5"	1' - 7"	1' - 10''	2' - 1"	2' - 3"
psi	General	Length	Tension	1' - 3"	1' - 6"	1' - 10"	2' - 2"	2' - 5"	2' - 9"	3' - 1"	3' - 7"
4000psi	Gen	Tension	Lap	1' - 7"	2' - 0"	2' - 5"	2' - 9"	3' - 2"	3' - 7"	4' - 0''	4' - 8''
= 4(		Compression Lap		1' - 3"	1' - 7"	1' - 11"	2' - 3"	2' - 6"	2' - 10"	3' - 3"	3' - 7"
Гc	Top Bar	Development Length Tension Lap		1' - 7"	2' - 0"	2' - 5"	2' - 9"	3' - 2"	3' - 7"	4' - 0''	4' - 8''
	йЧ			2' - 1"	2' - 7"	3' - 1"	3' - 7"	4' - 1"	4' - 8"	5' - 2"	6' - 1"

#### AASHTO LRFD Bridge Design Specifications, 2018

For Clear Cover or Spacing not meeting the minimums, development and lap lengths will be calculated in Design.

Ерох	y Coa	ated Bars w/ (	Cover≥3	db and Cle		Grade 60					
			Bar Size	#4	#5	#6	#7	#8	#9	#10	#11
	Minimum Clear Cover			1.500 in	1.875 in	2.250 in	2.625 in	3.000 in	3.384 in	3.810 in	4.230 in
	Minimum Bar Spacing (C-C)			3.500 in	4.375 in	5.250 in	6.125 in	7.000 in	7.896 in	8.890 in	9.870 in
		Development Length	Comp	0' - 10"	1' - 0''	1' - 3"	1' - 5"	1' - 7"	1' - 10"	2' - 1"	2' - 3"
psi	General		Tension	1' - 6"	1' - 10''	2' - 2"	2' - 7"	2' - 11"	3' - 3"	3' - 8''	4' - 1"
4000psi	Gen	Tension I	_ap	1' - 11"	2' - 5"	2' - 10"	3' - 4"	3' - 9"	4' - 3"	4' - 10''	5' - 4"
4		Compression Lap		1' - 3"	1' - 7"	1' - 11"	2' - 3"	2' - 6"	2' - 10"	3' - 3"	3' - 7"
ر ۲	ar ar	Development Length		1' - 11"	2' - 5"	2' - 10"	3' - 4"	3' - 9"	4' - 3"	4' - 10"	5' - 4"
	문 편 Development Length F 편 Tension Lap		2' - 6"	3' - 1"	3' - 8"	4' - 4''	4' - 11"	5' - 6"	6' - 3''	6' - 11"	

For Clear Cover or Spacing not meeting the minimums, development and lap lengths will be calculated in Design.
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Ерох	Epoxy Coated Bars w/ Cover < 3db for Slabs and Approach Slabs Grade 60												
			Bar Size	#4	#5	#6		#5	#6				
		Minimum Cl	ear Cover	1.000 in	1.000 in	1.000 in		1.500 in	1.500 in				
	Mir	nimum Bar Spa	cing (C-C)	2.500 in	2.625 in	2.750 in		3.625 in	3.750 in				
		Development	Comp	0' - 10"	1' - 0"	1' - 3"		1' - 0''	1' - 3"				
psi	General	Length	Tension	1' - 10"	2' - 9"	3' - 9"		2' - 3"	2' - 9"				
4000ps	Gen	Tension	Lap	2' - 5"	3' - 6"	4' - 10''		3' - 0"	3' - 7"				
= 4(		Compression Lap		1' - 3"	1' - 7"	1' - 11"		1' - 7"	1' - 11"				
fc	Top Bar	Development	Length	2' - 1"	3' - 1"	4' - 3"		2' - 7"	3' - 1"				
	Цщ	Tension	Lap	2' - 8"	4' - 0''	5' - 6"		3' - 4"	4' - 0''				

For Clear Cover or Spacing not meeting the minimums, development and lap lengths will be calculated in Design.

Common Stock Styles Of Welded Wire Fabric							
Style Designation	Spacing of Wires (in.)		Diameter of Wires (in.)		Sectional Area (in <sup>2</sup> / ft)		Weight (Lbs / 100 ft <sup>2</sup> )
	Long.	Trans	Long.	Trans	Long.	Trans	100 11-)
6 X 6-W1.4 X W1.4	6	6	0.134	0.134	0.028	0.028	21
6 X 6-W2 X W2	6	6	0.160	0.160	0.040	0.040	29
6 X 6-W2.9 X W2.9	6	6	0.192	0.192	0.058	0.058	42
6 X 6-W4 X W4	6	6	0.226	0.226	0.080	0.080	58
4 X 4-W1.4 X W1.4	4	4	0.134	0.134	0.042	0.042	31
4 X 4-W2 X W2	4	4	0.160	0.160	0.060	0.060	43
4 X 4-W2.9 X W2.9	4	4	0.192	0.192	0.087	0.087	62
4 X 4-W4 X W4	4	4	0.226	0.226	0.120	0.120	86

Welded wire fabric shall be selected from the following table.

Each structural unit (i.e., abutment, bent, deck, approach slab) uses a numerical **PREFIX** to separate bars in one unit from another. A note is placed on each sheet containing a Bill of Reinforcement identifying the bar prefix. The reinforcing steel fabricator shall prefix each bar in that unit to allow for easy identification of the bars at the job site. Generally, bars are prefixed in the following order: abutments, bents/piers, concrete girders, slab, and approach slabs.

Bundled bars may be used to provide the design steel requirements and should be detailed as follows.



The bar marks, number of bars required, and the bending diagrams for every bar in each structural unit are shown in a **BILL OF REINFORCEMENT** for each structural unit. Example bills are shown towards the end of this section.

Bars for each location shall be grouped numerically by bar size. Within each bar size, bars shall be listed with bent bars first, straight bars second, and set bars third. Bent bars and set bars shall be listed in ascending numerical order and straight bars shall be listed in ascending lengths. Plain bars, coated bars, and bars not included in the estimated quantity of reinforcing steel shall not be listed separately, but shall be combined within each bar size.

The weight shall be listed either under the location name or at the end of the list at each location. Weight is not required for approach slab bars or any other bars not included in the estimated quantity of reinforcing steel.

	BILL OF REI	NFORCEMENT			
Location	Mark	Number Required			
		Bent No. 1	Bent No. 2		
	5B2	27	27		
	5B3	24	24		
	536-6	10	10		
Сар	8B/	8	8		
	818-6	4	4		
	836-6	8	8		
	Weight	3152 LB	3/52 LB		
	4B4	2	2		
Columns	8/3-3	36	36		
	Weight	2385 LB	2385 LB		
	<u>[4B5</u> ]	2			
Drilled Shafts	<u>[4B6</u> ]		2		
	[824-10]	18			
	[827-10]		18		
	Bending	Diagrams			
36′-8″ ↓ ₩	لم الم عند الم	-8"	°℃ -** 2″-8*		
<u>881</u> (44'-10'')	(16'	(Tie) -8")	<u>5B3 (Tie)</u> (14'-8'')		
		rals			
Mark	Core	Pitch	Turns		
4B4	36″	2 <del>1</del> ″	<i>‡ 8/</i>		
[485]	36"	2 <del>1</del> ″	\$107		
[4B6]	36″	2ź"	\$/22		

BILL	OF REINFORCE	MENT
Location	Mark	Number Required
	455	140
	456	70
	*457	64
End	552	138
Diaphragms	535-0	16
	*653	104
	*Weight	*3051 LB
	Weight	3437 LB
	*45/	130
	*406-4	4
	*437-6	52
	*460-0	4
Slab and	*554	59
Curbs	*507-3	50
	*535-0	120
	*560-0	50
	*635-0	8
	*Weight	*12504 LB
	Bending Diagrams	
<u>2"</u> <u>1'-4"</u> <u>1'-6"</u> <u>*45/ (Tie)</u> (5'-7")	552 (5thrup) (5'-5'')	<u>15'-3"</u> ర్ష్మి <u>*653</u> (17'-3")
<u>4'-0"</u>	<u>3'-0"</u> <u>3'-9</u> " <u>*554</u> (36'-3"')	
455 (Stirrup) (2'-7")	456 (Stirrup) (2'-5")	2'-2" " " " " " " " " " " " " " " " " "

Number Required Per Pier

10

*1*-9-,1

3'-8"

<u>Set 2\_Bars</u> (Avg\_length = II'-O")

1'-8"

<u>5BI2 (T1a)</u> (11'-8")

4BI5 (Stirrup) (2'-6")

	BILL OI	F REINFORCEMEN	Τ		OF REINFORCE	IENT
Loca (Wei		Mark	Number Required	Location (Weight per Pier)	Mark	Num
		*451	46		4BI 3	
		4S2	62		525-8	
		504-5	13	Can	Set I Bars	
	(1046 LB)	*525-6	13	Cap (2332 LB)	Set 2 Bars	
		560-0	13		8BI4	
End Diaphragms		*653	41		812-0	
Siopin ogino		*451	42		925-8	<u> </u>
	Abut No. 2 (*2457 LB)	4S2	57		4BI5	
	(*2457 LB)	*525-6	13	Shaft	<u>4B</u> I6	
(94)	(941 LB)	557-8	13	(4031 LB)	410-0	
		*653	41		813-0	·]
Interm		4S6	56	Footing	706-8	
Diaphi (1195	rogms	504-3	10	Footing (4410 LB)	717-8	
(1195	18/	508-4	80		8BI7	
		*454	//2		Bending Diagrams	
		*455	115		λu	
		*411-2	41	11. 77. 1. 10		
		*460-0	41	/ <u>'~/</u> #″		
		*542-5	173	j j	-T	
		*558-5	59	Ň	<u>, 6'-10"</u>	
~	ab [	*560-0	59	ARIX	QDI A	41
\$1 (*386)		*Set 2 Bars	1	<u>4B 3</u> (5'-8")	<u>8814</u> (12'-8*')	
1 000		*Set 4 Bars	1	15-67		
		*604-10	4			
		*642-5	173			
		*657-8	4	9"R-"		e0-
		*660-0	4			
		*Set   Bars	1	<u>2'-0'</u>		-4*
		*Set 3 Bars	1	<u>4BI6 (Stir</u>		3 <u>BI7</u>
-		*454-6	2	(6'-4")	(7	"-4")
Curbs (*311 LB)		*457-11	2	Set Diagrams		
1.50	107	*460-0	4		Ser Didyi dilis	
		Bending Diagrams				-
3'-0	*	1'-3"		2-11-2	ု ဂို	P .
	]	5 <b></b>		× ×	ĥ	
e'-6"		, <del>,</del> ,	. 25'-0"		<u>''-8"</u> ''·	8"
ũ		I I	δ- <u>25-0</u>			(Tie)
1	I	<u>* * / / / / / / / / / / / / / / / / / /</u>	ស់	(8'-1/") (10	r-2") (10·	-4")
<b>*</b> 4SI (Sti		4S2 (Stirrup)	*653	Set I Bar	s	Set 2
(16'-0	<b>"</b> )	(4'-2")	(27'-0")	(Avg length = 9	<u>~</u> ~~6 <b>{</b> **}) //	vg leng
	- 1-			,	- c ·	
<del>1 //-</del>	<u>2#**</u> +	1-2#"-1				
-1 2″i-		- <u>+ 2</u> "+				
//·	-0#"	<u>I'-0</u> #"				
6 W	**	が ド	<u>ئ</u>			
킨	8	¥0	20.			
*	<u></u>	ì <b>l</b> #				
r-	22	1-2 ***	ليا 5*			
*4S	4 (Tie)	*4S5 (Tie)	4S6 (Stirrup)			
	-10")	(5'-0")	(12'-2")			
		Set Diagrams				
		Sor Digrama				
	<u>42'-4"</u> / s		42'-3"			
<b>.</b> т		BG Bars				
86 BG	18	u <u>r</u>				
BG Bars	1'-6*					
-T.	, 0	rs) *Set 2 R	ars (No. 5 Bars)			
*Set   E	Bars (No. 6 Ba	rs) *Set 2 Bo (Ava lend	ars (No. 5 Bars) ath = 21'-10 ±")			
*Set   E	, 0	ars) <u>*Set 2 Bo</u> ) (Avg leng	ars (No. 5 Bars) gth = 21'-10 ½")			
*Set   E (Avg	Bars (No. 6 Bo length = 21'-11" 42'-4"	ars) <u>*Set 2 Bo</u> ) (Avg leng	gth = 21'-10 ±") 48'-4"			
*Set   E (Avg	Bars (No. 6 Bo length = 21'-11" 42'-4"	<u>nrs) *Set 2 Bu</u> ) (Avg leny ស្តន្តាំ	gth = 21'-10 ±") 48'-4"			
*Set   E	Bars (No. 6 Ba length = 21'-11" 42'-4"	<u>rrs) <u>*Set 2 Bu</u> ) (Avg len</u>	gth = 21'-10 ±") 48'-4"			
*Set   E (Avg   E	Bars (No. 6 Ba ength = 21'-11" 	) (Avg len	$\frac{42'-4''}{1'-9''} = \frac{12}{3}$			
<u>*Set I E</u> (Avg I بر مربع *Set 3 م	Bars (No. 6 Bo length = 21'-11" 42'-4"	) (Avg len	gth = 21'-10 ±") 48'-4"			

### Cells

Name

REBAR REBARC

REBARL

REBARR

REBARU REBENT

REBEV

REBILL RECANE

RECRK4

RECRK5

RECRK6

REHOOK REPAT RESETB RESETS RESTPR RESTPU RETYBT RETYBV

RETYSQ

**Description** Section View of Rebar C Shaped Rebar L Shaped Rebar Barrier Rail Rebar U Shaped Rebar Bent Rebar Bevel Rebar Bill of Reinforcement Hook Rebar

1100111000
Crank Rebar for 4 Girders
Crank Rebar for 5 Girders
Crank Rebar for 6 Girders
Hook Rebar with Foot
Reinforcing Steel Pattern
Set Bent Rebar
Straight Rebar Cut Set
R Shaped Stirrup
U Shaped Stirrup
Bent Tie Rebar for Curb
Bevel Tie Rebar for Curb

Square Tie Rebar

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